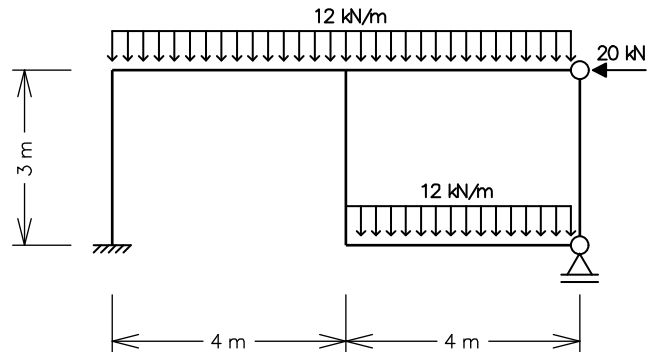


ENG 1204 - ANÁLISE DE ESTRUTURAS II - 2º Semestre - 2016

Prova Final - 21/12/2016 - Duração: 2:45 hs - Sem Consulta

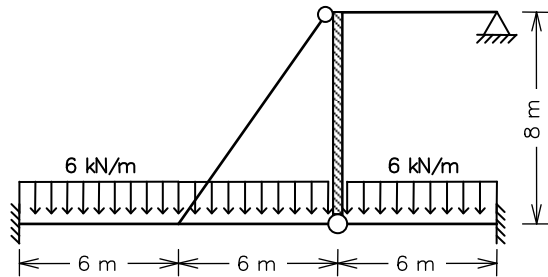
1ª Questão (5,0 pontos)

Determine pelo Método das Forças o diagrama de momentos fletores do quadro hiperestático ao lado. Somente considere deformações por flexão. Todas as barras têm a mesma inércia à flexão $EI = 7.2 \times 10^4 \text{ kNm}^2$.



2ª Questão (5,0 pontos)

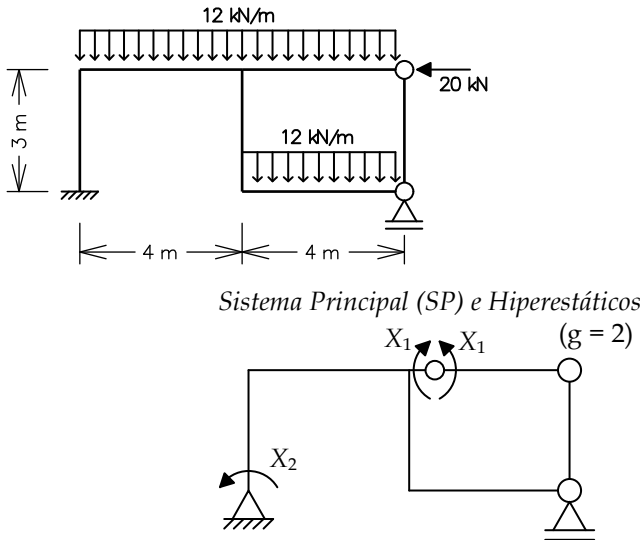
Empregando-se o Método dos Deslocamentos, obter o diagrama de momentos fletores para o quadro ao lado (barras inextensíveis). Todas as barras têm a mesma inércia à flexão $EI = 2.4 \times 10^4 \text{ kNm}^2$, com exceção da barra vertical que é infinitamente rígida à flexão.



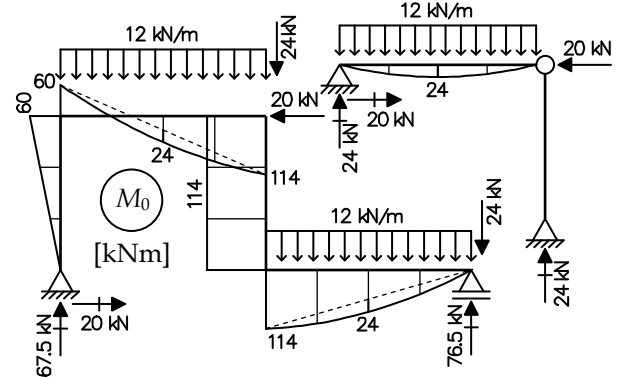
Solução de um sistema de 2 equações a 2 incógnitas:

$$\begin{Bmatrix} e \\ f \end{Bmatrix} + \begin{bmatrix} a & b \\ c & d \end{bmatrix} \begin{Bmatrix} X_1 \\ X_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} \Rightarrow \begin{cases} X_1 = \frac{bf - de}{ad - bc} \\ X_2 = \frac{ce - af}{ad - bc} \end{cases}$$

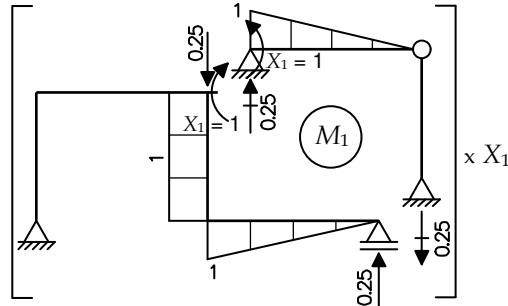
1ª Questão



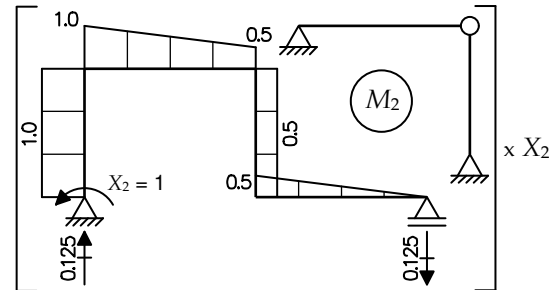
Caso (0) – Solicitação externa isolada no SP



Caso (1) – Hiperestático X1 isolado no SP



Caso (2) – Hiperestático X2 isolado no SP



Equações de compatibilidade:

$$\begin{cases} \delta_{10} + \delta_{11}X_1 + \delta_{12}X_2 = 0 \\ \delta_{20} + \delta_{21}X_1 + \delta_{22}X_2 = 0 \end{cases} \Rightarrow \frac{1}{EI} \begin{Bmatrix} +494 \\ -273 \end{Bmatrix} + \frac{1}{EI} \begin{bmatrix} +17/3 & -13/6 \\ -13/6 & +77/12 \end{bmatrix} \begin{Bmatrix} X_1 \\ X_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} \Rightarrow \begin{cases} X_1 = -81.4 \text{ kNm} \\ X_2 = +15.1 \text{ kN} \end{cases}$$

$$\delta_{10} = \frac{1}{EI} \cdot \left[-\frac{1}{3} \cdot 1 \cdot 24 \cdot 4 + 1 \cdot 114 \cdot 3 + \frac{1}{3} \cdot 1 \cdot 114 \cdot 4 + \frac{1}{3} \cdot 1 \cdot 24 \cdot 4 \right] = +\frac{494}{EI}$$

$$\delta_{20} = \frac{1}{EI} \cdot \left[+\frac{1}{3} \cdot 1.0 \cdot 60 \cdot 4 - \frac{1}{6} \cdot 1.0 \cdot 114 \cdot 4 + \frac{1}{6} \cdot 0.5 \cdot 60 \cdot 4 - \frac{1}{3} \cdot 0.5 \cdot 114 \cdot 4 - \frac{1}{3} \cdot 1.0 \cdot 24 \cdot 4 - \frac{1}{3} \cdot 0.5 \cdot 24 \cdot 4 \right. \\ \left. + \frac{1}{2} \cdot 1.0 \cdot 60 \cdot 3 - 0.5 \cdot 114 \cdot 3 - \frac{1}{3} \cdot 0.5 \cdot 114 \cdot 4 - \frac{1}{3} \cdot 0.5 \cdot 24 \cdot 4 \right] = -\frac{273}{EI}$$

$$\delta_{11} = \frac{1}{EI} \cdot \left[+2 \cdot \left(\frac{1}{3} \cdot 1 \cdot 1 \cdot 4 \right) + 1 \cdot 1 \cdot 3 \right] = +\frac{17}{3EI}$$

$$\delta_{12} = \delta_{21} = \frac{1}{EI} \cdot \left[-0.5 \cdot 1 \cdot 3 - \frac{1}{3} \cdot 0.5 \cdot 1 \cdot 4 \right] = -\frac{13}{6EI}$$

$$\delta_{22} = \frac{1}{EI} \cdot \left[+\frac{1}{3} \cdot 1.0 \cdot 1.0 \cdot 4 + \frac{1}{6} \cdot 1.0 \cdot 0.5 \cdot 4 + \frac{1}{6} \cdot 0.5 \cdot 1.0 \cdot 4 + \frac{1}{3} \cdot 0.5 \cdot 0.5 \cdot 4 \right. \\ \left. + 1.0 \cdot 1.0 \cdot 3 + 0.5 \cdot 0.5 \cdot 3 + \frac{1}{3} \cdot 0.5 \cdot 0.5 \cdot 4 \right] = +\frac{77}{12EI}$$

Momentos Fletores Finais:

$$M = M_0 + M_1 \cdot X_1 + M_2 \cdot X_2$$

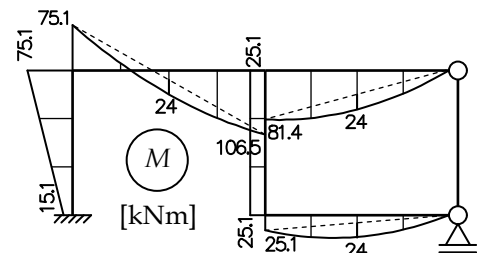


Diagram of a frame structure with a horizontal beam and a vertical column. The beam has a total length of 36m, divided into three 18m segments. It is subjected to a uniformly distributed load of 6 kN/m. The left end is fixed with a reaction moment of 18.0 kNm and a reaction of 18.0 kN. The right end is fixed with a reaction of 22.5 kN and a reaction moment of 27.0 kNm. A vertical column of height 6m is attached to the beam at 18m from the left. The column is subjected to a horizontal point load of 6 kN at the top. The top of the column is connected to a horizontal member that is fixed at the right end. The diagram shows the calculation of the fixed-end moment at the column base, β_{20} , using the principle of virtual work. The final result is $\beta_{20} = +67.5 \text{ kNm}$.

[illegible][illegible]
$$\begin{cases} \beta_{10} + K_{11}D_1 + K_{12}D_2 = 0 \\ \beta_{20} + K_{21}D_1 + K_{22}D_2 = 0 \end{cases} \Rightarrow \begin{Bmatrix} +9 \\ +67.5 \end{Bmatrix} + EI \cdot \begin{bmatrix} +22/15 & -1/6 \\ -1/6 & +1/12 \end{bmatrix} \cdot \begin{Bmatrix} D_1 \\ D_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} \Rightarrow \begin{cases} D_1 = -\frac{127.06}{EI} \\ D_2 = -\frac{1064.12}{EI} \end{cases}$$

Finais:

$+M_2 \cdot D_2$

M

[kNm]

